Ö REFERENCE

SEE SHEET 3 FOR PLAN SHEET LAYOUT AT TIME OF INVESTIGATION

CONTENTS

<u>LINE</u>	STATION	<u>PLAN</u>	PROFILI
-L-	10+00 TO 22+01.95	5 4	5
-YI-	10+00 TO 15+51.46	4	5
-Y2-	10+00 TO 11+65	4	5

APPENDICES

<u>APPENDIX</u>	<u>TITLE</u>	SHEETS		
Δ	SOIL LABORATORY RESULTS	6		

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

ROADWAY SUBSURFACE INVESTIGATION

COUNTY FORSYTH

PROJECT DESCRIPTION REPLACE BRIDGE NO. 95 OVER BLANKET CREEK ON SR 1100

INVENTORY

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	B-5152	1	6

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1991 707-680. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

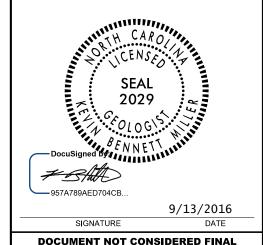
GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS MEDICATED DESCRIPTIONS AND ASSECTIONS OF THE INVESTIGATION. THE STATEM LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS MEDICATED DESCRIPTIONS AND ASSECTIONS AND ASSECTIONS OF THE INVESTIGATION. THE ACCORDING TO CLIMATIC CONDITIONS MEDICATED DESCRIPTIONS AND ASSECTIONS AND ASSECTIONS OF THE ACCORDING TO CLIMATIC CONDITIONS MEDICATED DESCRIPTIONS AND ASSECTIONS AND ASSECTIONS OF THE ACCORDING TO CLIMATIC CONDITIONS MEDICATED DESCRIPTIONS AND ASSECTIONS AND ASSECTIONS OF THE ACCORDING TO CLIMATIC CONDITIONS MEDICATED DESCRIPTIONS AS WELL AS A CRUPP NOW CHARTSE CACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- TES:
 THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.
 BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

	LENSONNEL
<u>J</u> .	K. STICKNEY
<i>C</i> .	. L. SMITH
R	. M. MOORE
INVESTIGATED BY	J. E. BEVERLY
DRAWN BY W. D	
CHECKED BY J. I	(, .
SUBMITTED BY	
	IBER, 2016

DEDCAMMEN



UNLESS ALL SIGNATURES COMPLETED

PROJECT REFERENCE NO.	SHEET NO.
B-5152	2

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS					
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL.	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.					
ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION		SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60	AQUIFER - A WATER BEARING FORMATION OR STRATA.					
		REPRESENTED BY A ZONE OF WEATHERED ROCK.	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.					
AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE,		ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING					
	ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.	WEATHERED NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES >						
CENERAL CRANIII AR MATERIALS SLIT-CLAY MATERIALS	MINERALOGICAL COMPOSITION	FINE TO COARSE CRAIN ICNEOUS AND METAMORPHIC ROCK THAT						
CLASS. (≤ 35% PASSING *200) (> 35% PASSING *200) ORGANIC MATERIALS	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC.	PROFY (CD) WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE,	SURFACE.					
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5		UNELSS, GABBRU, SCHIST, ELL	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.					
CLH35. A-1-6 A-1-6 A-2-4 A-2-5 A-2-6 A-2-7 A-7-6 H-3 H-6, H-7		NON-CRTSTALLINE SEDIMENTARY ROCK THAT WOULD YEILD SPT REFUSAL IF TESTED.	COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM					
SYMBOL 00000d00000		COASTAL PLAIN COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD						
% PASSING	## Company of the Com							
BAG 20 MY E0 MY E1 MN	PERCENTAGE OF MATERIAL							
*200 15 MX 25 MX 10 MX 35 MX 35 MX 35 MX 35 MX 36 MN 36 MN 36 MN 36 MN 36 MN	GRANULAR SILT - CLAY ORGANIC MATERIAL SOILS SOILS OTHER MATERIAL	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER						
MATERIAL		HAMMER IF CRYSTALLINE.						
			DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE					
PI 6 MX NP 10 MX 10 MX 11 MN 11 MN 10 MX 10 MX 11 MN 11 MN 11 MN MODERATE HIGHLY	March Marc							
GROUP INDEX 0 0 0 4 MX 8 MX 12 MX 16 MX NO MX AMOUNTS OF UNCANLL	GROUND WATER							
USUAL TYPES STONE FRAGS. FINE SILTY OR CLAYEY SILTY CLAYEY MATTER	▼ WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING							
UF MAJUR GRAVEL, AND SAND CRAVEL AND SAND SOLIS SOLIS	▼ STATIC WATER LEVEL AFTER 24 HOURS							
GEN BATING	─────────────────────────────────────	(MOD.) GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS						
AS SUBGRADE EXCELLENT TO GOOD FAIR TO POOR POOR UNSUITAB	.E.		FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.					
PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ;PI OF A-7-6 SUBGROUP IS > LL - 30	-							
CONSISTENCY OR DENSENESS	MISCELLANEOUS SYMBOLS							
PRIMARY COLL TYPE CUMPACINESS OR PENETRATION DECISIONES COMPRESSIVE STRENGTH	T ROADWAY EMRANKMENT (RE) 25/025 DIP & DIP DIRECTION							
CONSISTENCY (N-VALUE) (TONS/FT ²)		SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT	ITS LATERAL EXTENT.					
	SOIL SYMBOL SIPE INDICATOR SLOPE INDICATOR		LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.					
GRANULAR MEDIUM DENSE 10 TO 30 N/A	ADTIFICIAL FILL (AF) OTHER							
(NON-COHESIVE) DENSE 30 10 50								
VERT DENSE 7 30								
	Y		RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.					
(COHESIVE) VERY STIFF 15 TO 30 2 TO 4	TTTTT ALLUVIAL SOIL BOUNDARY A PIEZOMETER SPT N-VALUE							
		ROCK HARDNESS						
TEXTURE OR GRAIN SIZE		VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES	ROCK.					
Column C								
COARSE FINE	UCED IN THE TOD O FEET OF							
BOULDER COBBLE GRAVEL SAND SAND SILT CLAY								
(CSE, SU.) (F SU.)		HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED						
	Part							
	CL CLAY MOD MODERATELY γ - UNIT WEIGHT	HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE	WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL					
SOU MOISTURE SCALE FIELD MOISTURE	CSE COARSE ORG ORGANIC							
	DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST <u>SAMPLE ABBREVIATIONS</u>							
- SATURATED - USUALLY LIGHTON VERY WET LISHALLY	■		STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL					
(SAT.) FROM BELOW THE GROUND WATER TABLE			LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.					
DI ACTIC			TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.					
RANGE - WET - (W) SEMISULID; REDUIRES DRYING TO	FRAGS FRAGMENTS ω - MOISTURE CONTENT CBR - CALIFORNIA BEARING	FRACTURE SPACING BEDDING	BENCH MARK:					
PL + PLASTIC LIMIT								
OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE			ELEVATION: FEET					
		MODERATELY CLOSE 1 TO 3 FEET THINLY BEDDED 0.16 - 1.5 FEET	NOTES:					
REQUIRES ADDITIONAL WATER TO								
ATTAIN OPTIMUM MOISTURE	CMF-55	THINLY LAMINATED < 0.008 FEET						
PLASTICITY	8* HOLLOW AUGERS H							
	CME-550 HARD FACED FINGER BITS							
	TUNGCARBIDE INSERTS		FILE: B5152_LS_fin (DATED 05/11/2016)					
MODERATELY PLASTIC 16-25 MEDIUM	CASING W/ ADVANCER	CRAINC CAN BE CEDARATED FROM CAMPLE HITH CIFEL PRODE.						
	BORTARIE HOICE TRICONE STEEL TEETH							
COLOR								
		DIFFICULT TO BREAK WITH HAMMER.						
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).	CORE BIT VANE SHEAR TEST							
	VANE SHEAR IEST	SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; EXTREMELY INDURATED SAMPLE BREAKS ACROSS GRAINS.	DATE: 8-15-14					

See Sheet 1A-1 for Index of Sheets See Sheet 1B for Sheet Symbology S 51. B CITY LIMITS **PROIE** Elemmons PROJECT LOCATION VICINITY MAP

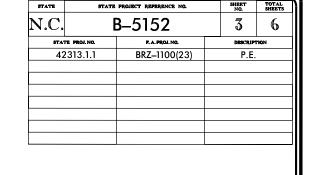
STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

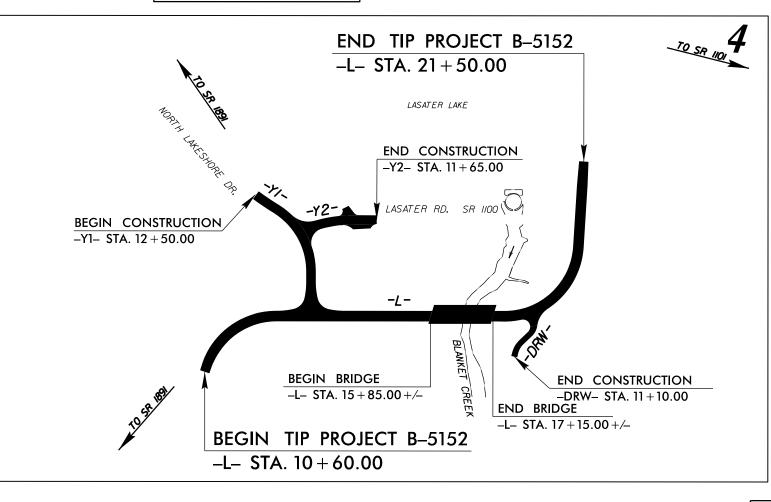
FORSYTH COUNTY

LOCATION: BRIDGE NO. 95 OVER BLANKET CREEK ON SR 1100

TYPE OF WORK: GRADING, DRAINAGE, PAVING, AND STRUCTURE

25% APPROVED PLANS





INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

GRAPHIC SCALES PROFILE (HORIZONTAL)

PROFILE (VERTICAL)

DESIGN DATA

K = 13 %D = 70 %

V = 25 MPH(TTST = 1% + DUAL = 2%)FUNC CLASS =

LENGTH ROADWAY STATE PROJECT B-5152 = 0.182 MILES LENGTH STRUCTURES STATE PROJECT B-5152 = 0.025 MILES TOTAL LENGTH STATE PROJECT B-5152 = 0.207 MILES

Prepared in the Office of: HDR Engineering, Inc. of the Carolinas 555 Fayetteville St., Suite 900 Raleigh, NC 27601 N.C.B.E.L.S. License Number: F-0116

012 STANDARD SPECIFICATIONS RIGHT OF WAY DATE: JANUARY 20, 2017

LETTING DATE: JANUARY 16, 2018

PHILLIP E. ROGERS, P.E.

ANTHONY G. THOMPSON, P.E.

REKHA PATEL, P.E.

HYDRAULICS ENGINEER

ROADWAY DESIGN **ENGINEER**

SIGNATURE:



THERE IS NO CONTROL OF ACCESS ON THIS PROJECT

THIS PROJECT IS WITHIN THE MUNICIPAL

BOUNDARIES OF THE VILLAGE OF CLEMMONS

ADT 2018 = 1340ADT 2038 = 1580

T = 3 %

LOCAL RURAL SUBREGIONAL TIER

PROJECT LENGTH



PAT McCRORY

NICHOLAS J. TENNYSON

Secretary

August 30, 2016

STATE PROJECT: 42313.1.1 (B-5152) FEDERAL PROJECT: BRZ-1100(23)

COUNTY: Forsyth

DESCRIPTION: Bridge No. 95 over Blanket Creek on SR 1100 (Lasater Rd.)

SUBJECT: Geotechnical Report—Inventory

This report presents the findings for the proposed roadway in conjunction with bridge No. 95 in Forsyth County. The bridge is to be relocated approximately 200 feet downstream (south) of the existing structure requiring a realignment of the 2 lane roadway connecting North Lakeshore Drive to Lasater Rd.

The geotechnical field investigation was conducted in the month of August 2016. An ATV mounted CME 550 drill machine with automatic hammer was utilized to perform 4 test borings along the proposed roadway realignment. Additionally, two vane shear tests were performed in the soft alluvial soils of the Blanket Creek floodplain. The following survey lines are addressed in this report.

Line	Station
-L-	10+60 to 21+50
-Y1-	10+00 to 12+50
-Y2-	10+00 to 11+65

Physiography / Geology:

This project is located in southwestern Forsyth County within the city limits of Clemmons. The area is surrounded by residential communities and wooded areas. Lasater Lake lies just to the north and drains into the Yadkin River south of the project area by way of Blanket Creek. Topography is flat to gently sloping with local elevations ranging between 675 and 710 feet.

Geologically the site lies within the Charlotte Belt with residual soil types derived from metamorphosed mafic rock types of Paleozoic Era (PzZm).

Areas of Special Geotechnical Interest:

- 1) Groundwater: Groundwater was encountered in all four borings, but static groundwater measurements were only taken in three. In all instances groundwater is more than 5 feet below proposed grade. However, it is worth noting that the water table within the Blanket Creek floodplain appears to lie within 2 feet of ground surface.
- 2) Alluvial Soils: The Blanket Creek floodplain is rather extensive and bisects the project between approximate L- stations 14+25 and 16+90. Alluvial soils consist of soft clayey sandy silt (A-4) with some organics and

very loose to loose silty clayey sand (A-2). Soil moisture content was noted to be 43.9% in the (A-4) strata at boring station 15+05, 17'RT –L-.

SHEET 3A

3) Crystalline Rock / Weathered Rock: Crystalline rock was not encountered during the course of this investigation. Weathered rock was noted at depth in two borings well below proposed grade.

Soil Properties:

- 1) *Fill Soils:* Roadway embankment fill soils are present beneath the existing roads. No boring data was obtained in existing roadway fill, however these soils appear well drained, stable, and are likely a combination of regional sands, silts and clays.
- 2) Alluvial Soils: Alluvial soils originate from water transportation and deposition in a floodplain environment. Alluvial deposits associated with this project were primarily encountered in the Blanket Creek floodplain at depths up to 11.1 feet. Alluvial soil types are described as soft, moist, clayey sandy silt (A-4) with some organics, and very loose to loose, moist to wet, silty clayey sand (A-2-4, A-2-6).
- 3) Residual Soils: These soils are derived from in place weathering of parent materials. They occur in a variety of consistencies, classifications, and stratigraphic sequences. Residual soils are further subdivided into clays, silts, and sands.

Clays are common soils within this region. They are found as surface soils and sub-soils. Clays specific to this site are described as stiff to hard, moist, silty sandy clay in the (A-7-5) AASHTO classification. Within the project corridor clay soils appear well drained and are medium plastic.

Silts are also common to the area and consist of soft to hard, moist, clayey sandy silt with the AASHTO classification of (A-4). Silts occur both near surface and at depth.

Sands may be found throughout the soil column as loose to very dense, moist, silty clayey sand in the (A-2-6) AASHTO classification.

Rock Properties:

1) Weathered Rock: Crystalline weathered rock was encountered in 2 boring locations. Weathered rock is defined as material exhibiting characteristics of the parent rock with SPT N-values of at least 100 blows per foot. Depth to weathered rock varied in each boring from 9 feet to 24 feet. No hard crystalline rock was encountered during our investigation.

Groundwater Properties:

Groundwater levels were measured immediately after drilling and again after a minimal 24 hour period to establish the static groundwater table. In some cases a static groundwater measurement may not be possible due to safety concerns.

For this project three of our four borings had static groundwater measurements. Depths to groundwater ranged from 2 to 6.1 feet.

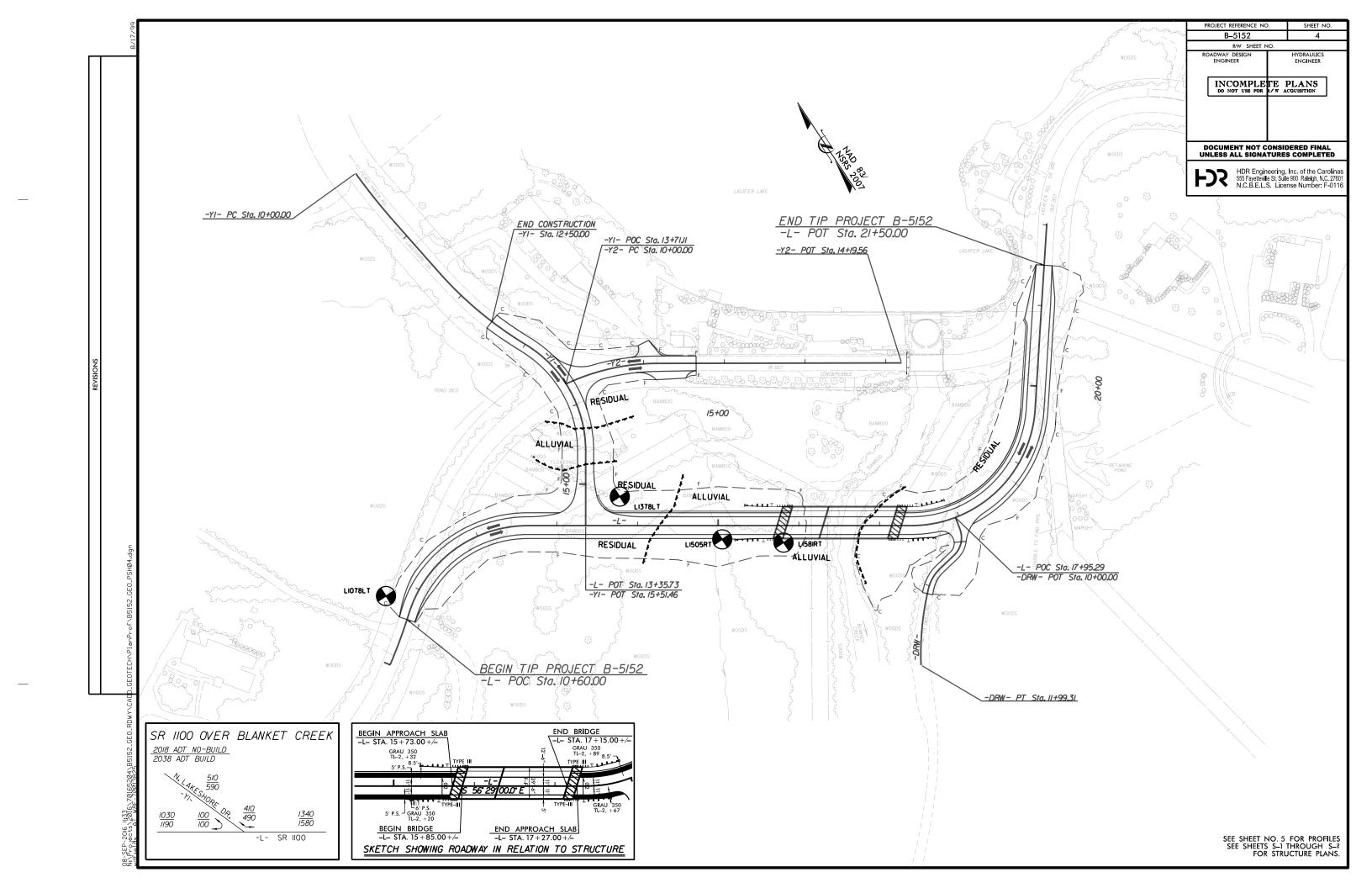
Undisturbed Samples:

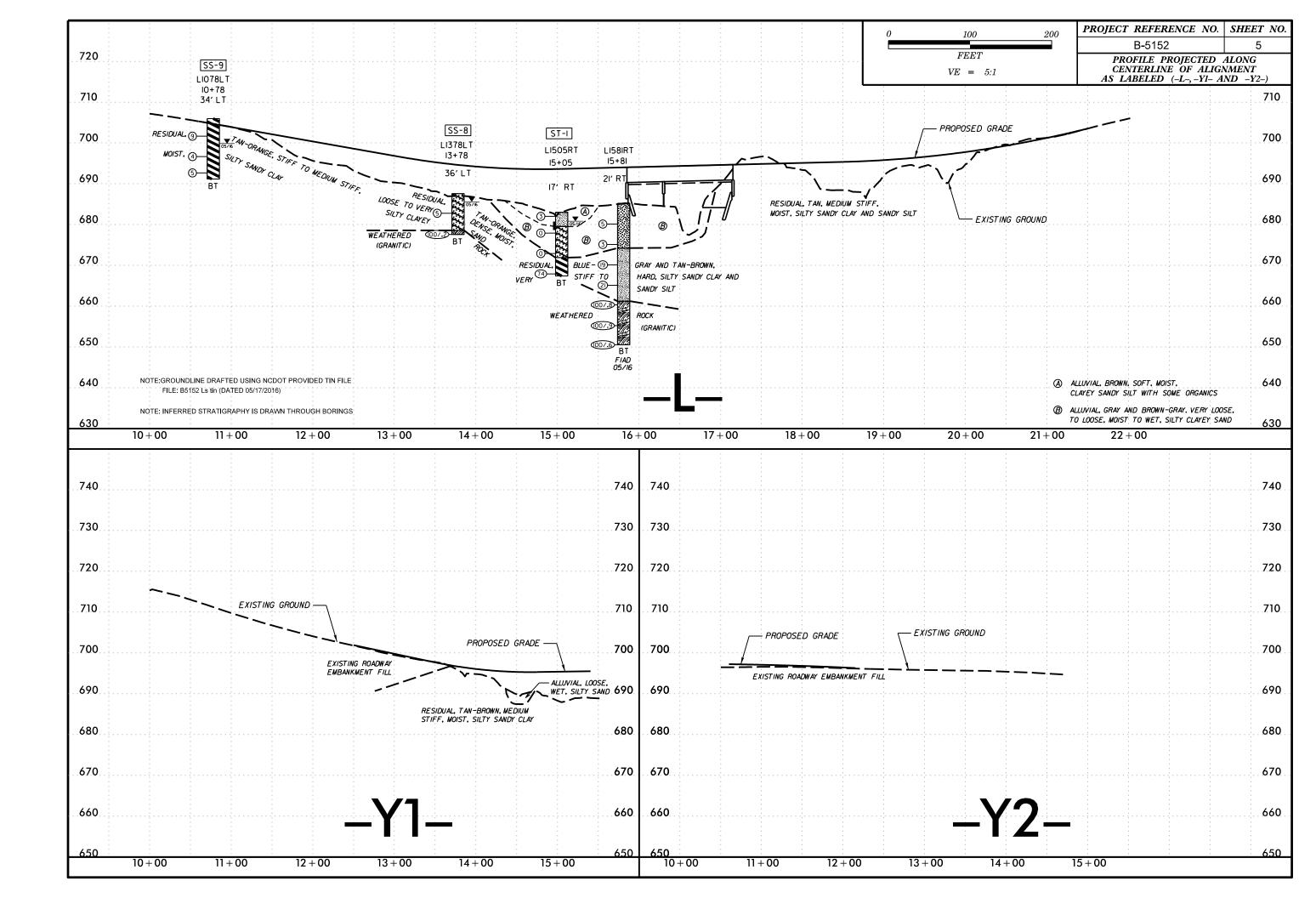
An undisturbed Shelby Tube sample was obtained from boring location at -L- station 15+05, 17' RT. Sample analysis was performed at the NCDOT Materials and Tests Unit.

Respectfully Submitted,

J. E. Beverly
Project Engineering Geologist

SHEET 3B 42313.1.1 (B-5152) Inventory





LABORATORY TESTING SUMMARY

PROJECT NUMBER:	42313.1.1	ID (TIP):	B-5152	COUNTY:	FORSYTH
_					
DESCRIPTION:	REPLACE BRIDGE NO. 95 OVER	R BLANKET CREEK ON SR 1100			

Sample No. Alignment		Station	0" 1	Depth	440::=0				% by V	Veight		%	%	Passing (sie	ves)		۵,
	Alignment		Offset (feet)	Interval (feet)	AASHTO Class.	L.L.	P.I.	Coarse Sand	Fine Sand	Silt	Clay	Retained #4 Sieve	#10	#40	#200	% Moisture	% Organic
SS-8	-L-	13+78	36' LT	4.3 - 5.3	A-2-6	N/A	N/A	26.2	28.0	11.6	34.2	-	100	87	49	-	
SS-9	-L-	10+78	34' LT	3.8 - 4.8	A-7-5(11)	54	16	19.1	19.5	21.0	40.3	-	98	87	64	-	
ST-1	-L-	15+05	17' RT	2.3 - 4.3	A-4(1)	28	5	7.0	41.1	27.9	24.0	-	100	98	60	43.9	
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